

Recycle Ponds Inflow Design Flood Control System Plan

Independence Steam Electric Station Newark, Independence County, Arkansas

October 2018 Revised October 2021

Prepared For:

Entergy Arkansas, LLC Independence Plant 555 Point Ferry Road Newark, Arkansas 72562

Prepared By:

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Revision History

Revision Number	Revision Date	Section Revised	Summary of Revisions
0	10/24/2018		Initial issue.
1	10/12/2021	1 through 6	Updated text and figures for five-year periodic revision.



1.0 Introduction

Entergy Arkansas, LLC (Entergy) operates the Independence Steam Electric Station (Plant). The purpose of this Inflow Design Flood Control System Plan (Plan) is to present the flood control features of the two Recycle Ponds, East and West (Ponds), as required by the United States Environmental Protection Agency's (USEPA) final coal combustion residual (CCR) rule Title 40 Code of Federal Regulations (40 CFR) Part 257 Subpart D - "Standards for the Disposal of Coal Combustion Residuals in Landfills and Surface Impoundments." The requirements for Inflow Design Flood Control System Plans for surface impoundments are presented in 40 CFR § 257.82. The initial Inflow Design Flood Control System Plan was developed in October 2018 (TRC) and placed in the Plant's operating record. The periodic inflow design flood control system plan revisions are required every 5 years pursuant to 40 CFR § 257.82(c)(4).

This Plan is revised based on review of the initial Plan, review of design documents, and a site visit by TRC to observe existing conditions.

1.1 Documents Reviewed

To develop this plan the following documents were reviewed by TRC:

- Initial Inflow Design Flood Control System Plan (TRC, 2018)
- Entergy's Independence Steam Electric Station Bottom Ash Work Plan. Document SD 1204.05A. Rev 2. November 10, 2008.
- Outfall Locations and Drainage Plan developed by Woodward Clyde Consultants, File No. 92T156C-A106. Rev 1. August 23, 2016.
- Topographic Survey performed by B&F Engineering, Inc. October 4, 2018.
 - Recycle Ponds Plan View. Drawings EX 1 and EX 2
- Notice of Intent to Initiate Closure of the West Recycle Pond
- Notice of Intent to Initiate Closure of the East Recycle Pond

1.2 Existing Conditions

The Plant is located at 555 Point Ferry Rd, Newark, Independence County, Arkansas (Figure 1). The Ponds are located on the southern side of the Plant. The East and West Ponds have an approximate surface area of 7.0 acres each. The Ponds operated with a typical water level elevation of approximately 235 feet (ft) North American Vertical Datum of 1988 (NAVD88). The top of berm elevation ranges between elevation 238 ft and 239 ft NAVD 88. Topography surrounding the immediate vicinity of the Ponds is generally flat-lying, with existing ground surface elevations ranging from approximately 235 to 239 ft NAVD88. Natural topography in the vicinity of the ponds is relatively flat to gently sloping.

Water contained in the Ponds is part of the Plant's bottom ash transport system. There are storm water swales around the Ponds to control storm water and limit the volume of run-on. The natural ground beyond the ponds gently slopes to the southwest. A topographic survey completed by



B&F Engineering, Inc. in October 2018 confirmed the elevations of the top of ground surface surrounding the ponds and the other surrounding areas.

Closure of West and East Ponds was initiated in August 2020 and February 2021, respectively. The closure plan for the Ponds indicates that closure will be performed by dewatering, removal of the CCR, and grading the former Pond area so that stormwater is not impounded. At the time of the site visit, the Ponds had been dewatered and removal of CCR was in progress.



2.0 Inflow Design Flood Control

40 CFR § 257.82 specifies requirements for hydrologic and hydraulic capacity of CCR units. The purpose of this report is to demonstrate the hydrologic and hydraulic capacity of the Ponds meet the requirement of the CCR Rule. Per ERM's Report, the Ponds are incised, therefore, a hazard potential classification is not needed per 40 CFR § 257.73(a). Based on the incised potential classification, the CCR surface impoundment unit must adequately manage flow into the unit during and following the peak discharge of the 25-year flood (40 CFR § 257.82(a)(3)(iv)).

The Ponds were designed and utilized as part of the Plant's bottom ash transport water system. The design meets the requirements of 40 CFR § 257.82(a)(1), (2) and (3), which states:

- The inflow design flood control system must adequately manage flow into the CCR unit during and following the peak discharge of the inflow design flood specified in paragraph (a)(3) of this section.
- (2) The inflow design flood control system must adequately manage flow from the CCR unit to collect and control the peak discharge resulting from the inflow design flood specified in paragraph (a)(3) of this section.
- (3) The inflow design flood is:
 - (i) For a high hazard potential CCR surface impoundment, as determined under § 257.73(a)(2) or §257.74(a)(2), the probable maximum flood;
 - (ii) For a significant hazard potential CCR surface impoundment, as determined under § 257.73(a)(2) or §257.74(a)(2), the 1000-year flood;
 - (iii) For a low hazard potential CCR surface impoundment, as determined under § 257.73(a)(2) or §257.74(a)(2), the 100-year flood;
 - (iv) For an incised CCR surface impoundment, the 25-year flood.

The Flood Insurance Rate Map (Federal Emergency Management Agency, 2010) indicates that the Ponds are located in an area mapped in Zone A of the special flood hazard areas subject to inundation by the 1 percent annual chance flood, refer to Appendix A. Figures 1 and 2 show the extents of the 100-year flood plain. Zone A is an area on the Flood Insurance Rate Map where the base flood elevation has not been determined. Therefore, to estimate the elevation of the 25-year flood for this Plan, an interpolation was performed using the nearest upstream and downstream locations where the flood elevations had been determined in the Flood Insurance Study (Federal Emergency Management Agency, 2012). This estimate resulted in a 25-year flood elevation 235 in the vicinity of the Ponds. Based on this result and the survey information, the Ponds are assumed to be above the elevation of the 25-year flood; therefore, a dedicated system to control flood waters is not required. The Ponds have been designed with inflow features presented in the sections below to mitigate and control stormwater inflows during a 25-year storm event.

In addition to the flood control requirements above, 40 CFR § 257.82(b) addresses discharges:

• Discharge from the CCR unit must be handled in accordance with the surface water requirements under 40 CFR §257.3-3.



The Ponds were designed and utilized as part of the Plant's bottom ash transport water system and comply with the requirements of 40 CFR § 257.82(b). Water discharges from the Plant are permitted through the Arkansas Department of Environmental Quality (ADEQ) for the National Pollutant Discharge Elimination System (NPDES).

2.1 Recycle Ponds Operation

The Ponds are utilized as part of the Plant's bottom ash transport water system. Site infrastructure enables the Ponds to be operated either singularly, in parallel, or in series. The Ponds can be drained via pumps in a pump house located between the ponds with a 30-inch intake pipe from each pond. The Ponds were designed to operate with two feet of freeboard height difference from the top of the surrounding ground surface to the design water level. In the event that both Ponds reach capacity, overflow from the Ponds flows through a culvert and ditch located in the northwest corner of the West Pond. The ditch discharges to the surge pond which is permitted to receive discharge from the Ponds in accordance with the NPDES permit.

The following control measures are implemented during operation to control the water levels in the Ponds:

- Operate pumps as needed to control the pond water levels.
- Regularly check and maintain grades surrounding the ponds to minimize the area contributing to storm water run-on.

2.2 Run-On Control System

As designed, the Ponds meet the requirement to control the storm water run-on from a 25-year, 24-hour storm event based upon the Precipitation Frequency Estimates from the National Oceanic and Atmospheric Administration. The perimeter berm surrounding the ponds reduces the run-on volume during precipitation events. The stormwater run-on volume calculated for the design storm was compared to the storage capacity above the Ponds design operating elevation. The evaluation determined that there is sufficient capacity in the Ponds when operating with two feet of freeboard to accept run-on volume from a 25-year, 24-hour storm event, refer to Appendix B.

2.3 **Pumping Capacity**

The pump station is equipped with six (three per unit) Gould single stage centrifugal pumps with a rated pumping capacity of 2,750 gallons per minute (gpm) at 307 feet of head in the LP Ash Water Pump house. A calculation was performed to determine the length of time required to remove the anticipated run-on due to a 25-year, 24-hour storm event, refer to Appendix B. The calculation was performed assuming only two operational pumps and resulted in a required time of 10 hours, or approximately 0.5 days, to remove the anticipated storm water run-on collected in both Ponds. This calculation shows that pumping rates are sufficient in controlling water levels in the Ponds. The peak discharge from the Ponds is dependent on the pump capacity which is controlled by the Plant.



2.4 Conclusions

The Ponds meet the requirements of 40 CFR § 257.82 of adequately controlling the inflows and outflows of peak discharge at the Plant for the following reasons:

- The Ponds are located above the 25-year flood elevation.
- The Ponds were adequately designed to manage precipitation from a 25-year, 24-hour storm event.
- The pumping rates are sufficient to manage the water levels in the Ponds in a controlled manner.
- The discharge from the Ponds is permitted under a NPDES permit which was issued in accordance with the provisions of the Federal Clean Water Act.



3.0 Notifications

In accordance with 40 CFR § 257.105(g), Entergy will post to the Plant's Facility Operating Record (FOR) the revised Inflow Design Flood Control System Plan. The Director of the ADEQ will be notified when documents are available as per 40 CFR § 257.106(g), and notices and documents will be placed on Entergy's CCR website consistent with 40 CFR § 257.107(g).

TRC | Entergy Arkansas, LLC Inflow Design Flood Control System Plan - Recycle Ponds Independence Steam Electric Station, Newark, Arkansas Final October 2018 Revised October 2021



4.0 Amendment and Periodic Plan Revision

In accordance with 40 CFR § 257.82(c)(2), Entergy may amend this Inflow Design Flood Control System Plan at any time. Specifically, Entergy will amend the inflow design flood control system plan whenever there is a change in the operation of the CCR unit that would substantially affect the written plan in effect.

A periodic inflow design flood control system plan must also be prepared every 5 years from the completion date of this plan.



5.0 References

- B&F Engineering, Inc. 2018. Topographic Survey: Independence Recycle Ponds Entergy Arkansas. October 4, 2018. Drawing.
- ERM. 2018. Summary of Site Visit and Review of CCR Structural Integrity Criteria Requirements for Entergy Independence Steam Electric Station (ISES) Recycle Pond. October 2018.
- Federal Emergency Management Agency. 2010. Flood Insurance Rate Map: Independence County, Arkansas and Incorporated Areas. Panel 400 of 550. Map Number 05063C0400D. Effective Date March 17, 2010. National Flood Insurance Program. Washington, D.C.
- Federal Emergency Management Agency. 2012. Flood Insurance Study: Independence County, Arkansas and Incorporated Areas. Flood Insurance Study Number 05063CV000B. Revised March 15, 2012. United States Department of Homeland Security. Washington, D.C.
- TRC. 2018. Recycle Ponds Initial Inflow Flood Control System Plan: Independence Steam Electric Station, Newark, Independence County, Arkansas. October 2018.



6.0 Certification

I, the undersigned Arkansas Professional Engineer, hereby certify that I am familiar with the technical requirements of 40 CFR § 257.82. I also certify that it is my professional opinion that, to the best of my knowledge, information, and belief, that the information in this inflow design flood control system plan is in accordance with current good and accepted engineering practice(s) and standard(s) and meets the technical requirements of 40 CFR § 257.82(c).

For the purpose of this document, "certify" and "certification" shall be interpreted and construed to be a "statement of professional opinion." The certification is understood and intended to be an expression of my professional opinion as an Arkansas Licensed Professional Engineer, based upon knowledge, information, and belief. The statement(s) of professional opinion are not and shall not be interpreted or construed to be or a warranty of the analysis herein.



<u>Michael J. Amstadt, P.E.</u> Name 14474

Engineer License Number

Signature of Professional Engineer

10/12/2021

Date

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Appendix A: Flood Information

- Flood Insurance Rate Map
- 25-Year Flood Elevation Estimate



Flood Insurance Rate Map

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (IPFE) and/or **flood**ways have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stilwater Elevations tables contained within the Flood Insurance Study (IFS) report that accompanies this FIRM. Users should be aware that EFEs shown on the FIRM represent rounded whole-toot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood eevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floocplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.07 North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report or this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used to construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood insurance Program. Floodway widths and other pertirent floodway data are provided in the Flood insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Arkansas State Plane north zone (FIPSZONE 0201). The borizontal datum was NAD83, GRS1980 spheroid. Differences in datum, spheroid, projection or State Plane zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in mag features across jurisdiction boundaries. These differences do not affect the accuracy of the FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 any elevation and the structure of 1988, visit the National Geodetic Gurvey website art http://www.gl. Datum of 1988, visit the National Geodetic Survey website art http://www.gl. Datum of visit.

NGS Information Services NOAA, N/NGS12 National Geodetic Survey SS/NC- 3, #9202 1315 East-West Highway Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the information Services Branch of the National Geodetic Survey at (301) 713-3242, or visit its website at http://www.ngs.noaa.gov/.

Base map information shown on the FIRM was provided in digital format by the Arkansas Geographic Information Office, 2007.

This map reflects more detailed and up to-date **stream channel configurations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profites and Floodway Data tables in the Flood Profites and Floodway Data tables and Floodway Data tables

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the ourtry showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flock Insurance Program dates for each community as well as a Isting of the panels on which each community is located.

Contact the FEMA Map Service Center at 1-800-358-9616 for information on available products associated with this FIRM. Available products may include pervocusity issued Letters of Map Change, a Flood insurance Study report, and/or digital versions of this map. The FENA Map Service Center may also be reached by Fax at 1-800-366 sel20 and its weeks at http://www.msclema.gov/

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please cal1-877-FEMA MAP (1-877-336-2627) or visit the FEMA website at http://www.fema.gov/.



		LEGEND		
	SPECIAL F	LOOD HAZARD AREAS (SFHAs) SUBJECT TO		
0.00*	The 1% annual chance flood	(100-year flood), also known as the base flood, is the flood		
45'00.00*	that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas			
	of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.			
³⁹ 57 ^{000m} N	ZONE A No Base Flood Elevations determined.			
	ZONE AE Base Flood Elevations determined. ZONE AH Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood			
	ZONE AO Flood depths	ermined. of 1 to 3 feet (usually sheet flow on sloping terrain);		
	average depth also determine	is determined. For areas of alluvial fan flooding, velocities d.		
	ZONE AR Special Flood chance flood	Hazard Area formerly protected from the 1% annual by a flood control system that was subsequently		
39	decertified. Zi being restore	one AR indicates that the former flood control system is d to provide protection from the 1% annual chance or		
56 N	ZONE A99 Area to be	protected from 1% annual chance flood by a Federal		
	flood protects determined.	on system under construction; no Base Flood Elevations		
	ZONE V Coastal flood Elevations det	zone with velocity hazard (wave action); no Base Flood ermined.		
	ZONE VE Coastal flood Elevations dete	zone with velocity hazard (wave action); Base Flood ermined.		
	FLOODWAY	AREAS IN ZONE AE		
³⁹ 55 ^{000m} N	The floodway is the channel	of a stream plus any adjacent floodplain areas that must be		
	kept free of encroachment se substantial increases in floo	o that the 1% annual chance flood can be carried without id heights.		
	OTHER FLO	OD AREAS		
	ZONE X Areas of 0.2	% annual chance flood; areas of 1% annual chance flood		
	1 square mi	depths of less than 1 foot or with drainage areas less than le; and areas protected by levees from 1% annual chance		
39000m				
54 N		AS		
20115 4	ZONE D Areas in which	h flood hazards are undetermined, but possible.		
LUILA	COASTAL E	ARRIER RESOURCES SYSTEM (CBRS) AREAS		
	OTHER HER			
	OBRS areas and OPAs areas	smally located within or adjacent to Special Floori Hazani Areas		
³⁹ 53 ^{000m} N		Floodplain boundary		
		Floodway boundary Zone D boundary		
		CBRS and OPA boundary		
		Boundary dividing Special Flood Hazard Areas of different		
	~~~~ 513 ~~~~~	Base Food Elevation line and value; elevation in feet*		
³⁹ 52 ^{000m} N	(EL 987)	Base Flood Elevation value where uniform within zone;		
52 14	* Referenced to the North Ame	elevation in feet" rican Vertical Datum of 1988 (NAVD 88)		
	AA	Cross section line		
	23	Transect line		
	97'07'30', 32'22'30'	Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)		
	4275 ^{000m} N	1000-meter Universal Transverse Mercator grid ticks, zone 15		
³⁹ 51 ^{000m} N	6000000 FT	5000-foot grid ticks: Arkansas State Plane coordinate system, north zone (FIPSZONE 0301), Lambert Conformal Conic		
	DX5510	Bench mark (see explanation in Notes to Users section of this FIRM panel)		
	• M1.5	River Mile		
	Bet	MAP REPOSITORIES ar to Map Repositories list on Map Index		
³⁹ 50 ^{000m} N	EFFECTIVE DATE OF COUNTYWDE			
		FLOOD INSURANCE RATE MAP March 17, 2010		
	EFFECTIV	E DATE(S) OF REVISION(S) TO THIS PANEL		
39 40 ^{000m} N	For community map revision	history prior to countywide mapping, refer to the Community		
49 N	To determine if flood insur	the Hood Insurance Study report for this jurisdiction.		
	agent or call the National Fig	ood Insurance Program at 1-800-638-6620.		
		MAP SCALE 1" = 2000'		
	1000	0 2000 4000 FEET		
³⁹ 48 ^{000m} N	600	0 600 1200		
	NICID	DANIEL GAGOD		
	NFIF	PANEL 0400D		
		FIRM		
³⁹ 47 ^{000m} N	2	FLOOD INSURANCE RATE MAP		
47 14	U U			
	ŏ	INDEPENDENCE COUNTY,		
	20	ARKANSAS		
		AND INCORPORATED AREAS		
90. 000				
- ³⁹ 46 ^{000m} N		PANEL 400 OF 550		
		(SEE MAP INDEX FOR FIRM PANEL LAYOUT)		
	2	CONTAINS:		
	R	INDEPENDENCE COUNTY 050090 0400 D		
	್ರಾ	MAGNESS, TOWN OF 050482 0400 D NEWARK, CITY OF 050092 0400 D		
- ³⁹ 45 ^{000m} N	<b>é</b>	OIL TROUGH, TOWN OF 050093 0400 D SULPHUR ROCK, TOWN OF 050290 0400 D		
-	0			
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	9			
		Notice to User. The Map Number shown below should be		
39000m		used when placing map orders; the Community Number shown above should be used on insurance applications for the subject		
- 44 N	V.			
	NIC	05063C0400D		
35°37'30.00" 2°30.00"		EFFECTIVE DATE		
		MARCH 17, 2010		
	NN NN	Federal Emergency Management Agency		
		reaction convergency management Agency		



**25-Year Flood Elevation Estimate** 



# INDEPENDENCE COUNTY, ARKANSAS AND INCORPORATED AREAS

Community Name	Community Number
BATESVILLE, CITY OF	050091
CAVE CITY, CITY OF	050313
CUSHMAN, TOWN OF	050403
MAGNESS, TOWN OF	050482
MOOREFIELD, TOWN OF	050483
NEWARK, CITY OF	050092
OIL TROUGH, TOWN OF	050093
PLEASANT PLAINS, TOWN OF	050484
SULPHUR ROCK, TOWN OF	050290
INDEPENDENCE COUNTY (UNINCORPORATED AREAS)	050090



REVISED: March 15, 2012



Federal Emergency Management Agency

FLOOD INSURANCE STUDY NUMBER 05063CV000B









## **Appendix B: Storm Water Calculations**

- Storm Water Run-On Estimate
- Pumping Time Estimate



Storm Water Run-On Estimate

<b>TRC</b> 8550 United Plaza Blvd, Suite 502, Bd	aton Rouge, LA 70809 • www	v.TRCcompanies.com	SHEET 1 OF 2	
PROJECT / LOCATION: Entergy Independence Steam Electric Station - Newark, AR PROJECT / PROPOSAL NO.				
SUBJECT: Storm Water Capacity of West and East Recycle Ponds	419735.0000			
PREPARED BY: A. Sampson	DATE: 9/5/2018	FINAL 🗆		
CHECKED BY: J. Hotstream	DATE: 9/9/2018	REVISION 🗆		

<u>Purpose</u>: Determine if 2 feet of freeboard is capable of containing the runoff volume from the 25-year, 24-hour storm event

#### Methodology:

1.) Determine the storage capacity of 2 feet Freeboard

- Storage Capacity of West Pond between EL. 235' and EL. 237'

$S_{FB}$	Storage Volume at Freeboard (from attached HydroCAD model)
S _{TFB}	Total Freeboard Storage Volume

 $S_{FB} = 587,616 \text{ ft}^3$ 

- Storage Capacity of East Pond between EL. 235' and EL. 237'

Freeboard Storage Capacity (Volumes are from attached HydroCAD model)

 $S_{FB} = 533,824 \text{ ft}^3$ 

-Total Freeboard Storage Capacity of West Pond and East Pond Combined

$$S_{TFB} = 1,121,440 \text{ ft}^3$$

TRC 8550 United Plaza Bl	vd, Suite 502, Baton Rouge, LA 70809	• www.TRCcon	<i>upanies.com</i>	SHEET 2 OF 2
			PROJECT / PROPOSAL NO.	
SUBJECT: Storm Water Capacity of West and East Recycle Ponds			419735.0000	
PREPARED BY: A. Sampson	DATE: 9/5/2018	FINAL		
CHECKED BY: J. Hotstream	DATE: 9/9/2018	REVISION		

2.) Determine the storm water runon volume that flows into the basins from the 25-year, 24-hour storm event

 $V_R$ = Volume of Runon Area = 19.8 ac Rainfall Rate 6.35 Design storm data from NOAA, refer to attached sheets = in  $V_R$ = Area * Rainfall Rate 456,400 ft³  $V_R$ =

3.) Compare Freeboard Capacity to Volume of Runoff to determine if the Freeboard is capable of containing 100-year, 24-hour storm event.

- If  $S_{TFB} > V_R$ , then the Freeboard design is OK

<u>Conclusion</u>: Because the  $S_{TFB} > V_R$ , the 2 feet of freeboard is capable of containing the runoff volume of the 25-year, 24-hour storm event



SEE SURVEYOR'S NOTE #2	<section-header></section-header>
<ul> <li><u>CENERAL NOTES:</u></li> <li>1. THIS SURVEY REPRESENTS A TOPOGRAPHIC SURVEY OF THE INDEPENDENCE RECYCLE PONDS.</li> <li>2. COORDINATES ARE NADB3(2011) ARKANSAS NORTH ZONE (0301 US SURVEY FEET) PROVIDED BY ENTERGY. ELEVATIONS ARE BASED ON FOUND MONUMENT SC-1 (LOCAL CONTROL).</li> <li>3. FIELD SURVEY WAS PERFORMED ON JULY 9TH, 10TH, 11TH, AND AUGUST 9TH, 2018.</li> </ul>	TOPOGRAPHIC SURVEY 555 POINT FERRY ROAD, NEWARK, AR DEPENDENCE RECYCLE PONDS NTY ARAN
LEGEND	ZOOBYDATEDesignJDrawnRLS7/18CheckedJSurveyTAWFld.Bk. #2177Rev. #J
80 50 60 20 0 40 80 160 SCALE: 1" = 80' REV. # REV. DATE BY REVISIONS	B&F PROJ. 7-4183-0201 FILE NAME: 001-FINAL ISSUE DATE: 10/01/18 SCALE: 1"=80'

,



### Area Listing (all nodes)

0.000	0	TOTAL AREA
(acres)		(subcatchment-numbers)
Area	CN	Description

### Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.000	Other	
0.000		TOTAL AREA

## Independence Recycle Ponds

Prepared by TRC				
HydroCAD® 10.00-20	s/n 01402	© 2017 HydroCAD	Software	Solutions LL

### Ground Covers (all nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
0.000	0.000	0.000	0.000	0.000	0.000	TOTAL AREA	

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Pond E: East Pond

Peak Elev=0.00' Storage=0 cf

Pond W: West Pond

Peak Elev=0.00' Storage=0 cf

### Summary for Pond E: East Pond

[43] Hint: Has no inflow (Outflow=Zero)

Volume	Invert	Avail.Storage	Storage	Description	
#1	235.00'	834,622 cf	Custom	Stage Data (Pr	ismatic)Listed below (Recalc)
Elevation	Surf./	Area In	c.Store	Cum.Store	
(feet)	(s	q-ft) (cub	vic-feet)	(cubic-feet)	
235.00	253,	568	0	0	
236.00	264,	923 2	259,246	259,246	
237.00	284,	234 2	274,579	533,824	
238.00	317,	361 3	600,798	834,622	

### Summary for Pond W: West Pond

[43] Hint: Has no inflow (Outflow=Zero)

Volume	Invert	Avail.Storage	e Storage	Description	
#1	235.00'	907,352 c	of Custom	Stage Data (Pri	smatic)Listed below (Recalc)
Elevation	Surf.	Area I	nc.Store	Cum.Store	
(feet)	(	sq-ft) (cu	lbic-feet)	(cubic-feet)	
235.00	286	5,201	0	0	
236.00	293	5,688	289,945	289,945	
237.00	301	,655	297,672	587,616	
238.00	337	7,817	319,736	907,352	

Precipitation Frequency Data Server

NOAA Atlas 14, Volume 9, Version 2 Location name: Newark, Arkansas, USA* Latitude: 35.6717°, Longitude: -91.4017° Elevation: 238.17 ft** * source: ESRI Maps ** source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffery Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

#### PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration	Average recurrence interval (years)									
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	<b>0.415</b> (0.355-0.489)	<b>0.472</b> (0.403-0.556)	<b>0.564</b> (0.481-0.665)	<b>0.640</b> (0.544-0.756)	<b>0.744</b> (0.620-0.889)	<b>0.823</b> (0.677-0.990)	<b>0.902</b> (0.728-1.10)	<b>0.981</b> (0.773-1.21)	<b>1.09</b> (0.836-1.35)	<b>1.16</b> (0.884-1.46)
10-min	<b>0.608</b> (0.520-0.716)	<b>0.691</b> (0.591-0.814)	<b>0.826</b> (0.705-0.974)	<b>0.937</b> (0.797-1.11)	<b>1.09</b> (0.908-1.30)	<b>1.21</b> (0.992-1.45)	<b>1.32</b> (1.07-1.61)	<b>1.44</b> (1.13-1.77)	<b>1.59</b> (1.22-1.98)	<b>1.70</b> (1.29-2.14)
15-min	<b>0.741</b> (0.634-0.873)	<b>0.842</b> (0.720-0.993)	<b>1.01</b> (0.860-1.19)	<b>1.14</b> (0.972-1.35)	<b>1.33</b> (1.11-1.59)	<b>1.47</b> (1.21-1.77)	<b>1.61</b> (1.30-1.96)	<b>1.75</b> (1.38-2.15)	<b>1.94</b> (1 49-2 41)	<b>2.08</b> (1.58-2.61)
30-min	<b>1.08</b> (0.922-1.27)	<b>1.23</b> (1.05-1.44)	<b>1.47</b> (1.25-1.73)	<b>1.66</b> (1.41-1.97)	<b>1.93</b> (1.61-2.31)	<b>2.14</b> (1.76-2.57)	<b>2.35</b> (1.89-2.85)	storm	ir, 24-ho event	ur (80)
60-min	<b>1.42</b> (1.21-1.67)	<b>1.60</b> (1.37-1.89)	<b>1.90</b> (1.62-2.24)	<b>2.16</b> (1.83-2.55)	<b>2.51</b> (2.10-3.01)	<b>2.79</b> (2.30-3.36)	<b>3.06</b> (2.48-3.73)	<b>3.35</b> (2.65-4.13)	<b>3.73</b> (2.88-4.66)	<b>4.03</b> (3.06-5.06)
2-hr	<b>1.76</b> (1.52-2.05)	<b>1.98</b> (1.71-2.31)	<b>2.34</b> (2.02-2.73)	<b>2.65</b> (2.27-3.10)	<b>3.09</b> (2.61-3.67)	<b>3.43</b> (2.86-4.10)	<b>3.78</b> (3.09-4.58)	<b>4.15</b> (3.31-5.08)	<b>4.64</b> (3.62-5.76)	<b>5.03</b> (3.86-6.27)
3-hr	<b>1.98</b> (1.72-2.29)	<b>2.22</b> (1.92-2.57)	<b>2.61</b> (2.26-3.04)	<b>2.96</b> (2.55-3.44)	<b>3.45</b> (2.93-4.09)	<b>3.84</b> (3.22-4.58)	<b>4.25</b> (3.49-5.12)	<b>4.67</b> (3.75-5.70)	<b>5.25</b> (4.12-6.49)	<b>5.71</b> (4.41-7.09)
6-hr	<b>2.41</b> (2.11-2.76)	<b>2.70</b> (2.36-3.10)	<b>3.19</b> (2.78-3.66)	<b>3.61</b> (3.14-4.16)	<b>4.22</b> (3.62-4.96)	<b>4.71</b> (3.99-5.56)	<b>5.21</b> (4.33-6.23)	<b>5.75</b> (4.66-6.96)	<b>6.48</b> (5.14-7.95)	<b>7.05</b> (5.50-8.70)
12-hr	<b>2.92</b> (2.58-3.32)	<b>3.30</b> (2.91-3.75)	<b>3.93</b> (3.46-4.47)	<b>4.46</b> (3.92-5.09)	<b>5.22</b> (4.52-6.07)	<b>5.83</b> (4.97-6.81)	<b>6.44</b> (5.40-7.63)	<b>7.08</b> (5.80-8.50)	<b>7.96</b> (6.37-9.69)	<b>8.63</b> (6.80-10.6)
24-hr	<b>3.49</b> (3.11-3.92)	<b>3.97</b> (3.54-4.46)	4.76 (4.24-5.36)	<b>5.42</b> (4.81-6.12)	6.35 (5.54-7.29)	7.07 (6.08-8.17)	<b>7.79</b> (6.59-9.13)	8.54 (7.05-10.1)	<mark>9.53</mark> (7.70-11.5)	<b>10.3</b> (8.19-12.5)
2-day	<b>4.09</b> (3.69-4.55)	<b>4.67</b> (4.20-5.19)	<b>5.61</b> (5.04-6.24)	<b>6.38</b> (5.72-7.13)	<b>7.45</b> (6.56-8.46)	<b>8.28</b> (7.19-9.47)	<b>9.10</b> (7.76-10.6)	<b>9.94</b> (8.28-11.7)	<b>11.0</b> (9.00-13.2)	<b>11.9</b> (9.55-14.3)
3-day	<b>4.49</b> (4.07-4.96)	<b>5.11</b> (4.62-5.64)	<b>6.11</b> (5.52-6.76)	<b>6.94</b> (6.25-7.70)	<b>8.08</b> (7.15-9.12)	<b>8.96</b> (7.83-10.2)	<b>9.85</b> (8.45-11.4)	<b>10.7</b> (9.01-12.6)	<b>11.9</b> (9.78-14.2)	<b>12.8</b> (10.4-15.4)
4-day	<b>4.82</b> (4.38-5.30)	<b>5.45</b> (4.96-6.00)	<b>6.50</b> (5.90-7.16)	<b>7.37</b> (6.66-8.14)	<b>8.57</b> (7.61-9.63)	<b>9.49</b> (8.33-10.8)	<b>10.4</b> (8.98-12.0)	<b>11.4</b> (9.58-13.3)	<b>12.6</b> (10.4-15.0)	<b>13.6</b> (11.0-16.3)
7-day	<b>5.61</b> (5.14-6.12)	<b>6.30</b> (5.78-6.87)	<b>7.45</b> (6.81-8.14)	<b>8.41</b> (7.67-9.21)	<b>9.74</b> (8.74-10.9)	<b>10.8</b> (9.55-12.1)	<b>11.8</b> (10.3-13.5)	<b>12.9</b> (11.0-15.0)	<b>14.4</b> (12.0-17.0)	<b>15.5</b> (12.7-18.5)
10-day	<b>6.32</b> (5.83-6.85)	<b>7.06</b> (6.50-7.66)	<b>8.28</b> (7.61-9.00)	<b>9.31</b> (8.53-10.1)	<b>10.7</b> (9.69-11.9)	<b>11.9</b> (10.6-13.3)	<b>13.0</b> (11.4-14.8)	<b>14.2</b> (12.1-16.4)	<b>15.8</b> (13.2-18.6)	<b>17.0</b> (14.0-20.2)
20-day	<b>8.43</b> (7.85-9.04)	<b>9.30</b> (8.65-9.98)	<b>10.7</b> (9.97-11.5)	<b>11.9</b> (11.1-12.9)	<b>13.6</b> (12.4-15.0)	<b>14.9</b> (13.4-16.5)	<b>16.2</b> (14.4-18.3)	<b>17.6</b> (15.2-20.2)	<b>19.4</b> (16.4-22.7)	<b>20.8</b> (17.3-24.5)
30-day	<b>10.2</b> (9.57-10.9)	<b>11.2</b> (10.5-12.0)	<b>12.9</b> (12.1-13.8)	<b>14.3</b> (13.3-15.3)	<b>16.2</b> (14.8-17.7)	<b>17.6</b> (16.0-19.4)	<b>19.1</b> (17.0-21.4)	<b>20.6</b> (17.9-23.4)	<b>22.5</b> (19.1-26.1)	<b>23.9</b> (20.1-28.1)
45-day	<b>12.5</b> (11.8-13.2)	<b>13.8</b> (13.0-14.6)	<b>15.8</b> (14.9-16.8)	<b>17.5</b> (16.4-18.6)	<b>19.7</b> (18.1-21.3)	<b>21.4</b> (19.4-23.3)	<b>23.0</b> (20.5-25.5)	<b>24.5</b> (21.4-27.8)	<b>26.6</b> (22.7-30.7)	<b>28.0</b> (23.7-32.8)
60-day	<b>14.5</b> (13.7-15.2)	<b>16.0</b> (15.1-16.9)	<b>18.4</b> (17.4-19.4)	<b>20.3</b> (19.1-21.5)	<b>22.9</b> (21.1-24.5)	<b>24.7</b> (22.5-26.8)	<b>26.5</b> (23.7-29.2)	<b>28.1</b> (24.6-31.7)	<b>30.2</b> (25.9-34.7)	<b>31.7</b> (26.9-37.0)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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Precipitation Frequency Data Server

### **PF** graphical





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#### Maps & aerials

#### Small scale terrain

1 2

5 10 25

50 100

200 500

- 1000

2-day

3-day

4-day

7-day

10-day

20-day

30-day

45-day

60-day



Large scale terrain





Large scale aerial

Precipitation Frequency Data Server



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US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: <u>HDSC.Questions@noaa.gov</u>

**Disclaimer** 



**Pumping Time Estimate** 

TRC | Entergy Arkansas, LLC Inflow Design Flood Control System Plan - Recycle Ponds Independence Steam Electric Station, Newark, Arkansas

S550 United Plaza Blv	d, Suite 502, Baton Rouge, LA 70809 • a	www.TRCcompanies.com	SHEET 1 OF 1	
PROJECT / LOCATION: Entergy Independence Stea	PROJECT / PROPOSAL NO.			
SUBJECT: Estimated Pump Down Time	419735.0	419735.0000		
PREPARED BY: J. Bell	DATE: 8/27/2018	FINAL 🗆		
CHECKED BY: L Hotstream	DATE: 9/9/2018	REVISION 🛛		

<u>Purpose</u>: Determine the amount of time needed for the pump to remove the storm water collected during the 25-year, 24-hour storm event to design operation elevation (EL. 235)

Methodology:

1.) Use the volume of runon  $(V_R)$  from the Freeboard Volume Calculation (Refer to attached calculation sheet)

 $V_{R}$  = 456,400 ft³

2.) Use pump capacity rating to determine amount of time to lower the water level in both basins to EL. 235

-Assume one pump is operational for each pond

Pump Rate = 2,750 gallons per minute (gpm)

$$V_R$$
 = 3,414,109 gallons

Time = 
$$\frac{V_R}{(Pump Rate)^* 2}$$
  
Time = 621 min  
= 10.3 hr  
= 0.4 days

<u>Conclusion</u>: It will take approximately 10 hours to pump out the storm water to reestablish freeboard after the 25-year, 24-hour storm event.